

$$\begin{aligned} \text{stress} &= (0.95 \times 0.9)/1.4 - 1.4 \times 0.09 \\ &\quad \text{(proportionally reduced from (12.18))} \\ &= 0.61 - 0.126 = 0.484 \text{ N/mm}^2 \quad \text{(no tension)} \end{aligned} \quad (12.20)$$

- Leeward side

$$\text{dead} + \text{wind} = 1.4 G_k + 1.4 W_k \quad (12.21)$$

$$\text{stress} = 0.95 + 0.126 = 1.08 \text{ N/mm}^2 \quad (12.22)$$

(iii) Dead, imposed and wind loads

$$\text{dead} + \text{imposed} + \text{wind} = 1.2 G_k + 1.2 Q_k + 1.2 W_k \quad (12.23)$$

$$\begin{aligned} \text{stress} &= (0.95 \times 1.2)/1.4 + (1.2 \times 0.20)/1.6 \pm 1.2 \times 0.09 \\ &= 0.814 + 0.15 \pm 0.108 \\ &= 1.07 \text{ or } 0.856 \text{ N/mm}^2 \quad \text{(no tension)} \end{aligned} \quad (12.24)$$

In this case also the severe loading condition appears to be (12.17).

(d) *Third floor*

(i) Design and imposed loads

$$\begin{aligned} \text{design} + \text{imposed} &= 1.4 G_k + 1.6 Q_k \\ &= 1.4 \times 94.04 + 1.6 \times 15.12 \\ &= 131.66 + 24.19 = 155.85 \text{ kN/m} \end{aligned} \quad (12.25)$$

$$\begin{aligned} \text{stress} &= (131.66 \times 10^3)/(102.5 \times 10^3) \\ &\quad + (24.19 \times 10^3)/(102.5 \times 10^3) \\ &= 1.28 + 0.24 = 1.52 \text{ N/mm}^2 \end{aligned} \quad (12.26)$$

(ii) Dead and wind loads

- Windward side

$$\text{dead} + \text{wind} = 0.9 G_k + 1.4 W_k \quad (12.27)$$

$$\begin{aligned} \text{stress} &= (0.9 \times 1.28)/1.4 - 1.4 \times 0.162 \\ &= 0.823 - 0.227 = 0.596 \text{ N/mm}^2 \quad \text{(no tension)} \end{aligned} \quad (12.28)$$

- Leeward side

$$\text{dead} + \text{wind} = 1.4 G_k + 1.4 W_k \quad (12.29)$$

$$\text{stress} = 1.28 + 0.227 = 1.51 \text{ N/mm}^2 \quad (12.30)$$

(iii) Dead, live and wind loads

$$\text{dead} + \text{live} + \text{wind} = 1.2 G_k + 1.2 Q_k + 1.2 W_k \quad (12.31)$$

$$\begin{aligned} \text{stress} &= (1.28 \times 1.2)/1.4 + (0.24 \times 1.2)/1.6 \pm 1.2 \times 0.162 \\ &= 1.097 + 0.18 \pm 0.194 \\ &= 1.47 \text{ or } 1.08 \text{ N/mm}^2 \text{ (no tension develops)} \end{aligned} \quad (12.32)$$

The critical load combination is (12.25) and the load is 155.85 kN/m.

(e) *Second floor*

(i) Design and imposed loads

$$\begin{aligned} \text{design} + \text{imposed} &= 1.4 G_k + 1.6 Q_k \\ &= 1.4 \times 118.72 + 1.6 \times 16.2 \\ &= 166.2 + 25.9 = 192.10 \text{ kN/m} \end{aligned} \quad (12.33)$$

$$\begin{aligned} \text{stress} &= (1.4 \times 118.72 \times 10^3)/(102.5 \times 10^3) \\ &\quad + (1.6 \times 16.2 \times 10^3)/(102.5 \times 10^3) \\ &= 1.62 + 0.25 = 1.87 \text{ N/mm}^2 \end{aligned} \quad (12.34)$$

(ii) Dead and wind loads

● Windward side

$$\text{dead} + \text{wind} = 0.9 G_k + 1.4 W_k \quad (12.35)$$

$$\begin{aligned} \text{stress} &= (0.9 \times 1.62)/1.4 - (1.4 \times 0.253) \\ &= 1.04 - 0.35 = 0.69 \text{ N/mm}^2 \text{ (no tension)} \end{aligned} \quad (12.36)$$

● Leeward side

$$\text{dead} + \text{wind} = 1.4 G_k + 1.4 W_k \quad (12.37)$$

$$\text{stress} = 1.62 + 1.4 \times 0.253 = 1.97 \text{ N/mm}^2 \quad (12.38)$$

(iii) Dead, imposed and wind loads

$$\text{dead} + \text{imposed} + \text{wind} = 1.2 G_k + 1.2 Q_k + 1.2 W_k \quad (12.39)$$

$$\begin{aligned} \text{stress} &= (1.62 \times 1.2)/1.4 + (0.25 \times 1.2)/1.6 + 1.2 \times 0.253 \\ &= 1.39 + 0.19 \pm 0.30 \\ &= 1.88 \text{ or } 1.28 \text{ N/mm}^2 \text{ (no tension)} \end{aligned} \quad (12.40)$$